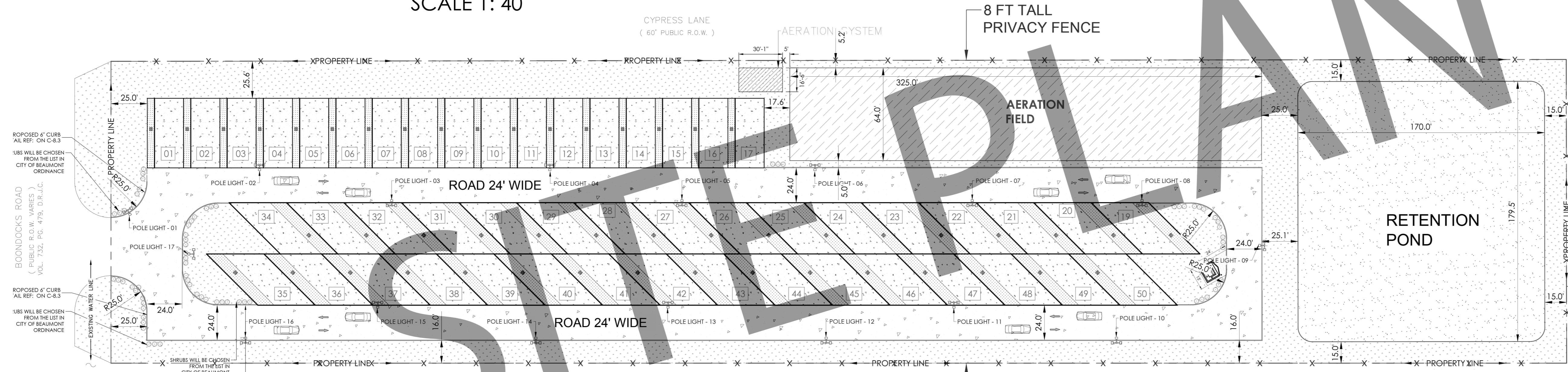
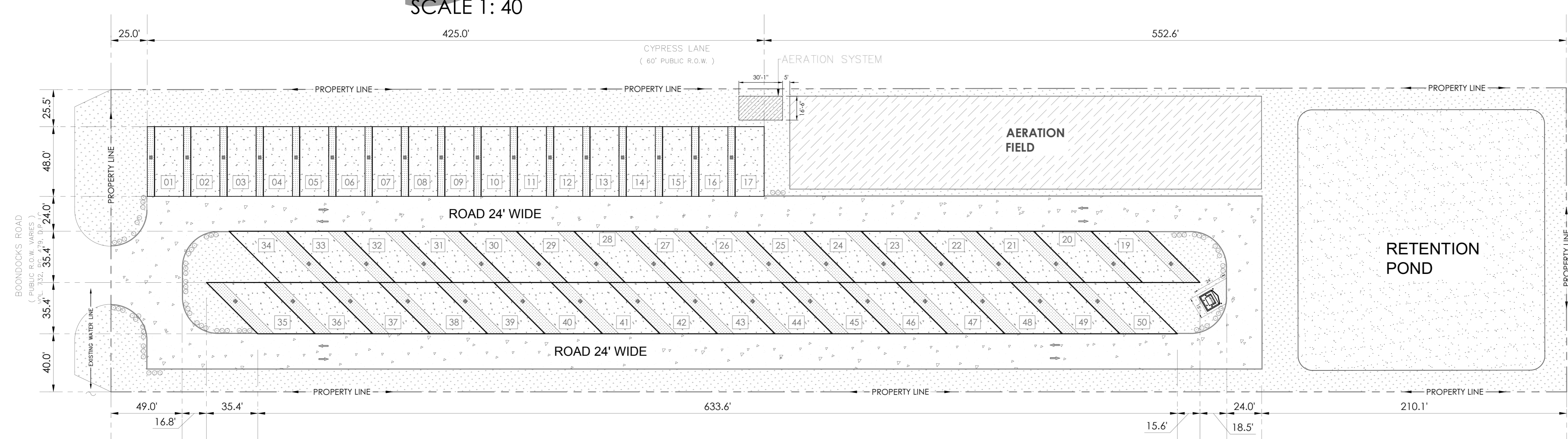


SITE PLAN
SCALE 1: 40



SITE PLAN
SCALE 1: 40



TOTAL PROVIDED RV SPOTS: 50

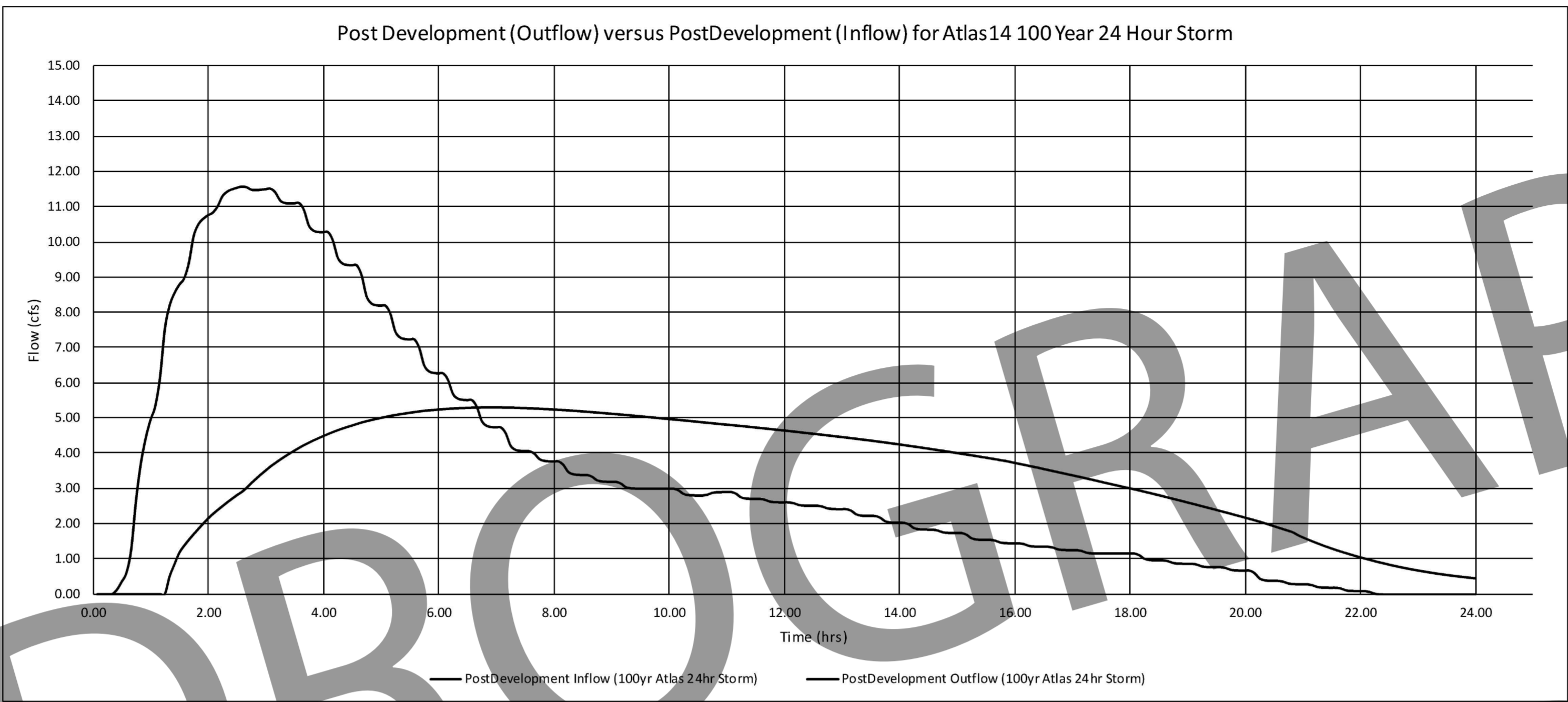
PROPOSED RV'S PARK
7111 CYPRESS LN, BEAUMONT, TX 77705



SITE PLAN

Duration	Precipitation (in)			Precipitation Depth - Partial Duration
	25-yr	10-yr	100-yr	
6-hr	8.56	6.62	12.10	
12-hr	10.40	7.90	15.10	
24-hr	12.30	9.26	18.10	
96-hr	16.40	12.30	24.50	

NOAA Atlas 14, Volume 11, Version 2									
Time (hh:mm)	Time (hr)	24-hr Duration							
		Precip. (%)	Cumul. Prec. (in)			Intensity (in/ hr)			
			25-yr	10-yr	100-yr	25-yr	10-yr	100-yr	
0:00	0.0	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0:30	0.5	1.80	0.22	0.17	0.33	0.44	0.33	0.65	
1:00	1.0	6.08	0.75	0.56	1.10	1.05	0.79	1.55	
1:30	1.5	11.83	1.46	1.10	2.14	1.41	1.06	2.08	
2:00	2.0	18.35	2.26	1.70	3.32	1.60	1.21	2.36	
2:30	2.5	25.14	3.09	2.33	4.55	1.67	1.26	2.46	
3:00	3.0	31.85	3.92	2.95	5.76	1.65	1.24	2.43	
3:30	3.5	38.24	4.70	3.54	6.92	1.57	1.18	2.31	
4:00	4.0	44.15	5.43	4.09	7.99	1.45	1.09	2.14	
4:30	4.5	49.49	6.09	4.58	8.96	1.31	0.99	1.93	
5:00	5.0	54.22	6.67	5.02	9.81	1.16	0.88	1.71	
5:30	5.5	58.37	7.18	5.41	10.56	1.02	0.77	1.50	
6:00	6.0	61.96	7.62	5.74	11.21	0.88	0.66	1.30	
6:30	6.5	65.07	8.00	6.03	11.78	0.77	0.58	1.13	
7:00	7.0	67.77	8.34	6.28	12.27	0.66	0.50	0.98	
7:30	7.5	70.13	8.63	6.49	12.69	0.58	0.44	0.85	
8:00	8.0	72.25	8.89	6.69	13.08	0.52	0.39	0.77	
8:30	8.5	74.19	9.13	6.87	13.43	0.48	0.36	0.70	
9:00	9.0	76.01	9.35	7.04	13.76	0.45	0.34	0.66	
9:30	9.5	77.74	9.56	7.20	14.07	0.43	0.32	0.63	
10:00	10.0	79.42	9.77	7.35	14.38	0.41	0.31	0.61	
10:30	10.5	81.07	9.97	7.51	14.67	0.41	0.31	0.60	
11:00	11.0	82.68	10.17	7.66	14.97	0.40	0.30	0.58	
11:30	11.5	84.24	10.36	7.80	15.25	0.38	0.29	0.56	
12:00	12.0	85.76	10.55	7.94	15.52	0.37	0.28	0.55	
12:30	12.5	87.20	10.73	8.07	15.78	0.35	0.27	0.52	
13:00	13.0	88.55	10.89	8.20	16.03	0.33	0.25	0.49	
13:30	13.5	89.82	11.05	8.32	16.26	0.31	0.24	0.46	
14:00	14.0	90.99	11.19	8.43	16.47	0.29	0.22	0.42	
14:30	14.5	92.06	11.32	8.52	16.66	0.26	0.20	0.39	
15:00	15.0	93.04	11.44	8.62	16.84	0.24	0.18	0.35	
15:30	15.5	93.94	11.55	8.70	17.00	0.22	0.17	0.33	
16:00	16.0	94.77	11.66	8.78	17.15	0.20	0.15	0.30	
16:30	16.5	95.54	11.75	8.85	17.29	0.19	0.14	0.28	
17:00	17.0	96.26	11.84	8.91	17.42	0.18	0.13	0.26	
17:30	17.5	96.93	11.92	8.98	17.54	0.16	0.12	0.24	
18:00	18.0	97.55	12.00	9.03	17.66	0.15	0.11	0.22	
18:30	18.5	98.12	12.07	9.09	17.76	0.14	0.11	0.21	
19:00	19.0	98.63	12.13	9.13	17.85	0.13	0.09	0.18	
19:30	19.5	99.07	12.19	9.17	17.93	0.11	0.08	0.16	
20:00	20.0	99.42	12.23	9.21	18.00	0.09	0.06	0.13	
20:30	20.5	99.67	12.26	9.23	18.04	0.06	0.05	0.09	
21:00	21.0	99.85	12.28	9.25	18.07	0.04	0.03	0.07	
21:30	21.5	99.94	12.29	9.25	18.09	0.02	0.02	0.03	
22:00	22.0	99.98	12.30	9.26	18.10	0.01	0.01	0.01	
22:30	22.5	99.99	12.30	9.26	18.10	0.00	0.00	0.00	
23:00	23.0	99.99	12.30	9.26	18.10	0.00	0.00	0.00	
23:30	23.5	99.99	12.30	9.26	18.10	0.00	0.00	0.00	
0:00	24.0	100.00	12.30	9.26	18.10	0.00	0.00	0.00	



Post-Development - Pipe Calculations

Pipe	Inlet Node	Outlet Node	Length	Diameter	Number of Pipes	Inlet Flowline Elevation	Outlet Flowline Elevation	Slope	Manning's Roughness	Area (Full)	Velocity (Full)	Capacity (Full)	10-Yr			100-Yr		
													Peak Flow	Peak Velocity	Maximum Depth	Peak Flow	Peak Velocity	Maximum Depth
P-1.0	A-1.0	A-2.0	364 ft	24 in	1	6.58 ft	6.04 ft	0.15%	0.0130	3.14 sq. ft	2.77 ft/s	8.71 cfs	1.96 cfs	1.46 ft/s	1.17 ft	4.10 cfs	1.57 ft/s	2.00 ft
P-2.0	A-2.0	Pond-1	256 ft	24 in	1	6.04 ft	5.58 ft	0.18%	0.0130	3.14 sq. ft	3.05 ft/s	9.59 cfs	4.71 cfs	3.26 ft/s	1.63 ft	9.55 cfs	3.96 ft/s	2.00 ft
Orifice-1	Pond-1	Outfall	62 ft	10 in	1	4.86 ft	4.74 ft	0.18%	0.0130	0.55 sq. ft	9.36 ft/s	5.10 cfs	3.26 cfs	5.98 ft/s	2.26 ft	4.98 cfs	9.14 ft/s	4.69 ft

Post-Development - Node Calculations

Node	Invert Elevation	Ground Elevation	Initial Water Surface Elevation	Surcharge Elevation	10-Yr		100-Yr	
					Maximum Water Surface Elevation	Peak Inflow	Maximum Water Surface Elevation	Peak Inflow
A-1.0	6.58 ft	9.50 ft	6.58 ft	10.00 ft	7.29 ft	1.96 cfs	9.70 ft	4.12 cfs
A-2.0	6.04 ft	9.50 ft	6.04 ft	10.00 ft	7.20 ft	4.71 cfs	9.67 ft	9.61 cfs

Pond Design

Pond Size	Depth (ft)	Elevation	Area (ft^2)	Volume (ft^3)	Notes
Top Area 2.85'	6.00	10.86	28,864	139,655.00	1' Freeboard
5' Depth (3:1)	5.00	9.86	26,927	111,850.00	100-Yr Max WSL
4' Depth (3:1)	4.00	8.86	25,049	85,925.00	
3' Depth (3:1)	3.00	7.86	23,222	61,830.00	
2' Depth (3:1)	2.00	6.86	21,454	39,511.00	
1' Depth (3:1)	1.00	5.86	19,745	18,919.00	24" Influent Pipe is at 5.58'
0' Depth	0.00	4.86	18,104	-	18" Effluent Pipe (w/ 10" Restrictor)

Pond Calculations

Storm Details	Max Inflow (cfs)	Inflow Volume (ft^3)	Predevelopment Q (cfs)	Storage Required (ft^3)	Max HGL Depth (Elevation) of Pond
Atlas14 - 10 Year 24-Hour Duration	5.65	146,590	2.25	43,583	2.26' (7.12)
Atlas14 - 100 Year 24-Hour Duration	11.57	300,399	5.10	80,044	4.69' (9.55)

PreDevelopment - Subbasin Hydrology

Predevelopment Drainage Area	Atlas14 10-Yr Storm	Atlas14 100-Yr Storm
	Sub-A	Sub-A
PreDevelopment Time of Concentration (Tc) - per SCS Method		
Sheet Flow: Tsh = (0.007 * ((n * Lf)^0.8)) / ((P^0.5) * (Sf^0.4))		
Manning's Roughness : n	0.25	0.25
Flow Length (ft) : Lf	100	100
Slope (ft/ft) : Sf	0.0035	0.0035
2 yr. 24 hr Rainfall (in) : P2	5.44	5.44
Computed Flow Time (min) : Tc	22.71	22.71
Shallow Concentrated Flow: Tsc = ((Lf/ V)/ (3600))*60		
Slope (ft/ft) : Sf	0.003	0.003
Velocity (unpaved) (ft/s): V=16.1345 * (Sf^0.5)	0.88	0.88
Flow Length (ft) : Lf	387	387
Computed Flow Time (min) : Tc	7.30	7.30
Computed Flow Time (min): Summation of Tc		
	30.01	30.01
Area (ac)	2.31	2.31
Total Rainfall (in)	9.26	18.10
Total Runoff (in)	7.16	15.82
Peak Runoff (cfs)	2.25	5.10
Weighted Curve Number	82.78	82.78

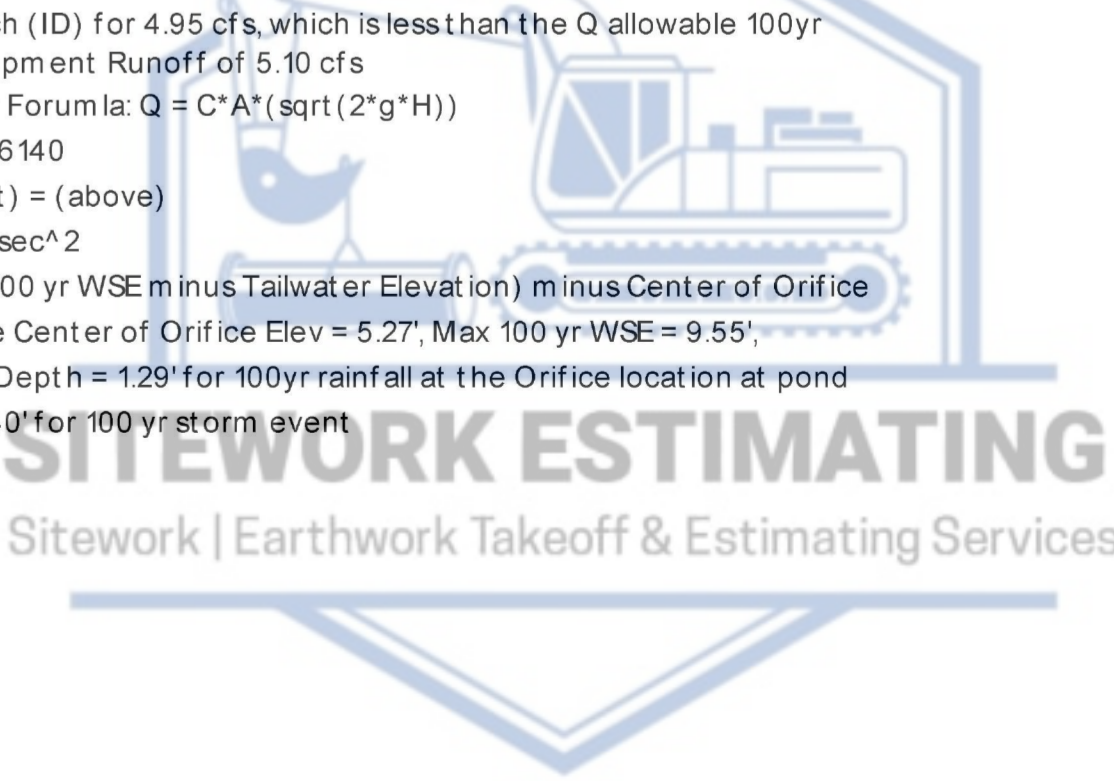
Post Development - Subbasin Hydrology

	Atlas14 10-Yr Storm			Atlas14 100-Yr Storm		
	A-1	A-2	A-3	A-1	A-2	A-3
Area (ac)	1.753	2.287	0.776	1.753	2.287	0.776
Total Rainfall (in)	9.26	9.26	9.26	18.1	18.1	18.1
Total Runoff (in)	7.95	8.53	8.62	16.72	17.35	17.44
Peak Runoff (cfs)	1.97	2.75	0.94	4.13	5.56	1.89
Weighted Curve Number	89.17	93.93	94.69	89.17	93.93	94.69
Time of Concentration (mins)	10.00	10.00	10.00	10.00	10.00	10.00

Outflow Size to Drainage Ditch

Restrict or Size (in)	Area (Sq ft)	Head (ft)	Flow, Q (cfs)
8	0.349	3.400	3.170
10	0.545	3.400	4.953
12	0.785	3.400	7.132

Use 10-inch (ID) for 4.95 cfs, which is less than the Q allowable 100yr Predevelopment Runoff of 5.10 cfs
Restrict or Formula: $Q = C * A * (\text{sqrt}(2 * g * H))$
Coeff = 0.6140
Area (sq ft) = (above)
 $g = 32.2 \text{ ft/sec}^2$
 $H \text{ (ft)} = (100 \text{ yr WSE minus Tailwater Elevation) minus Center of Orifice for 10" the Center of Orifice Elev} = 5.27', \text{ Max 100 yr WSE} = 9.55',$
Tailwater Depth = 1.29' for 100yr rainfall at the Orifice location at pond
Head = 3.40' for 100 yr storm event



T.B.M. #2
NAIL IN ASPHALT
N: 13882547.43
E: 3481905.25
ELEV: 8.38

24" RCP F/L = 4.60

24" RCP F/L = 4.63

FOUND 3" STEEL PIPE
N: 13882547.43
E: 3481938.16

18" FL EL=4.74'

24" RCP F/L = 4.76

24" RCP F/L = 4.85

24" RCP F/L = 4.72

FOUND 1" STEEL PIPE
N: 13882338.03
E: 3481949.46

24" RCP F/L = 4.90

24" RCP F/L = 4.91

PROP SWALE FL=6.00'

62 L.F. ~ 18" RCP
@ 0.18%

TYP. 3:1 SIDE SLOPES

18" FL EL=4.86' W/
10" RESTRICTOR, SEE
DETAIL THIS SHEET

A-3
0.776 AC

24" FL EL=5.58'

24" RCP F/L = 4.71

885 L.F. PROPOSED SWALE
SEE DETAIL THIS SHEET

PROPOSED RV'S (LINE-1)
6-RV SPOTS

256 L.F. ~ 24" HDPE
@ 0.15%

RIM EL=9.50'
24" FL EL=6.04'

PROPOSED RV'S (LINE-3)
15-RV SPOTS

PROPOSED RV'S (LINE-4)
15-RV SPOTS

A-2
2.287 AC

PROPOSED RV'S (LINE-2)
19-RV SPOTS

364 L.F. ~ 24" HDPE
@ 0.15%

RIM EL=9.50'
24" FL EL=6.58'

A-1
1.753 AC

ROAD 24' WIDE

PROPOSED PROPERTY LINE SWALE CROSS
SECTION, SEE DETAIL THIS SHEET

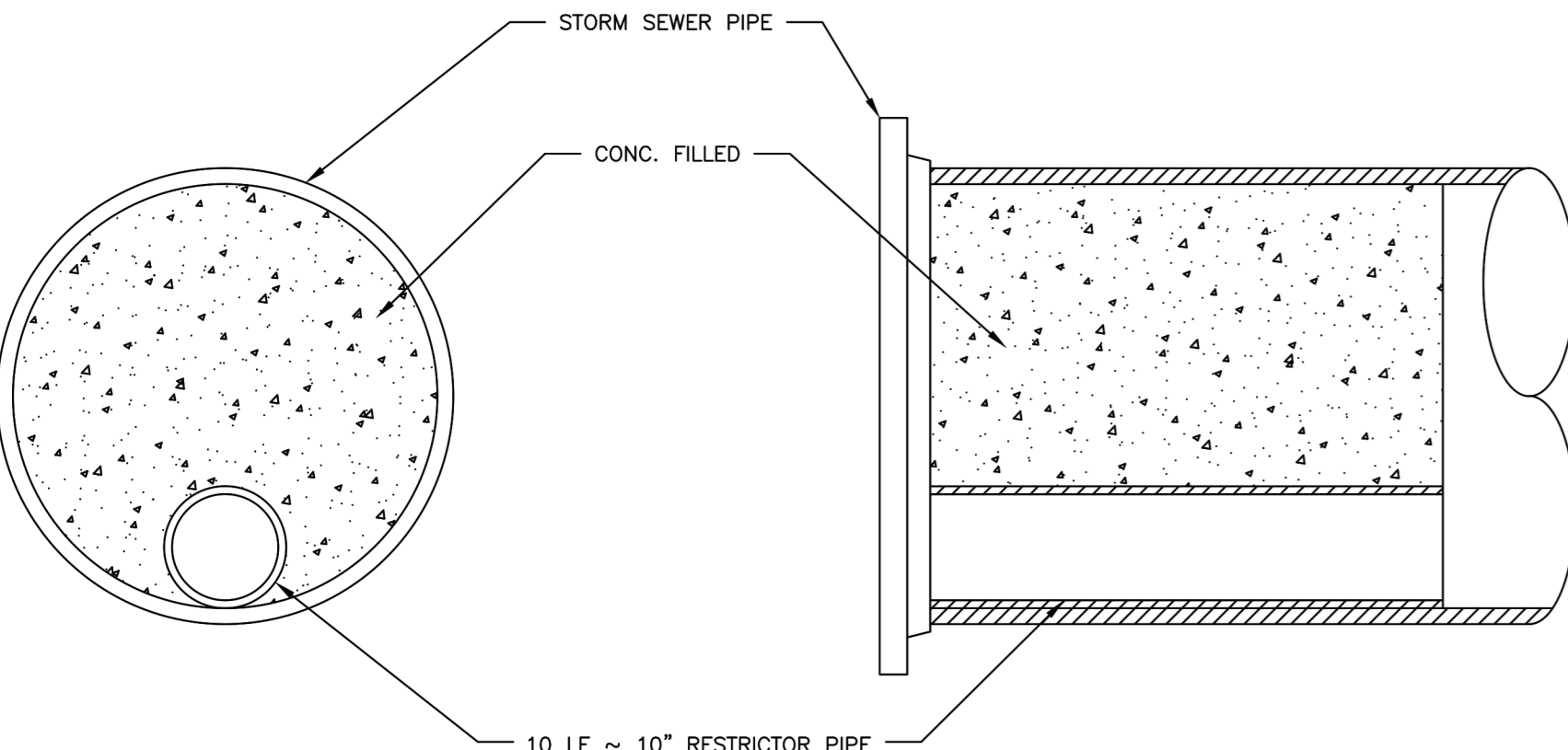
PROP SWALE FL=6.75'

AERATION
AREA

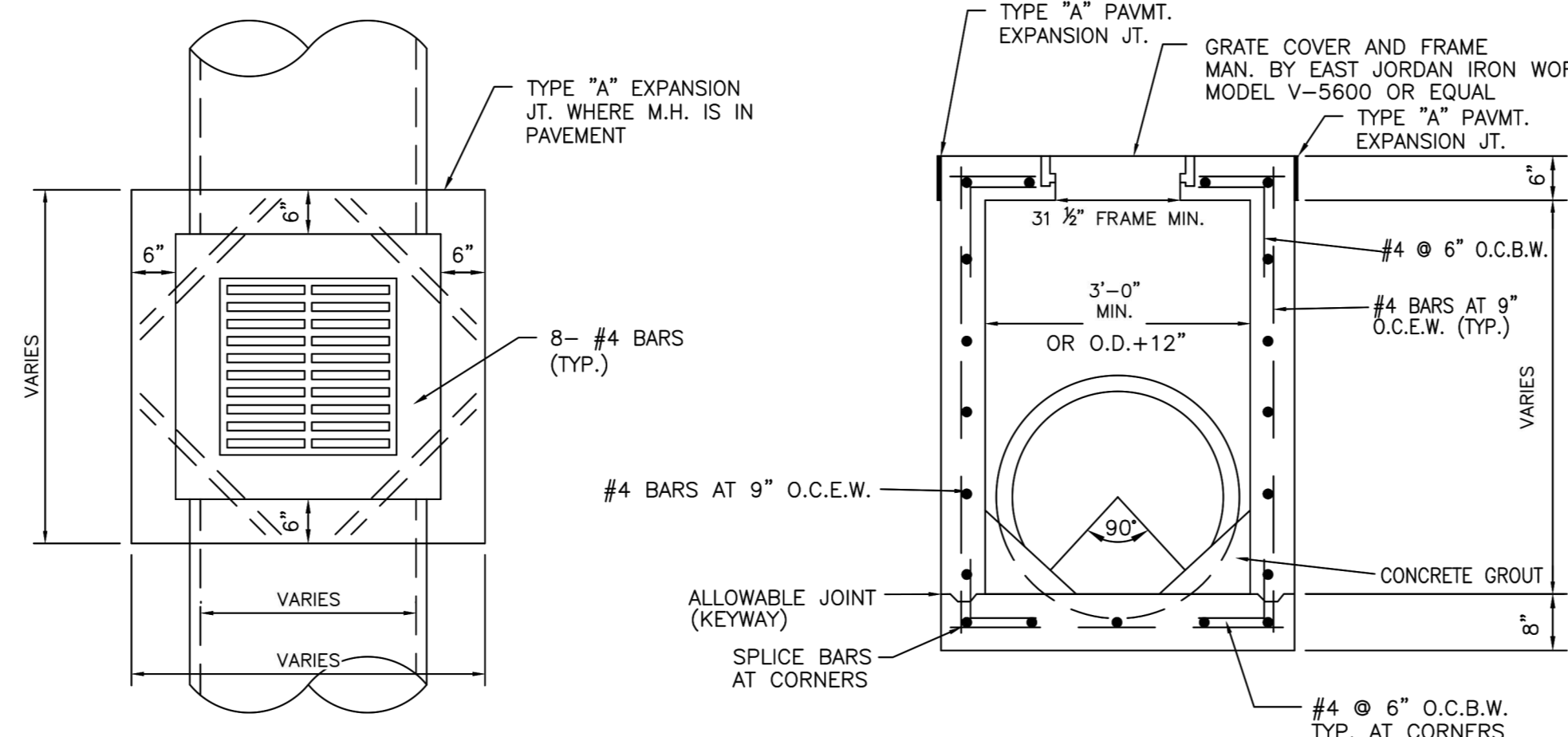
PROP SWALE FL=6.75'

DRAINAGE PLAN

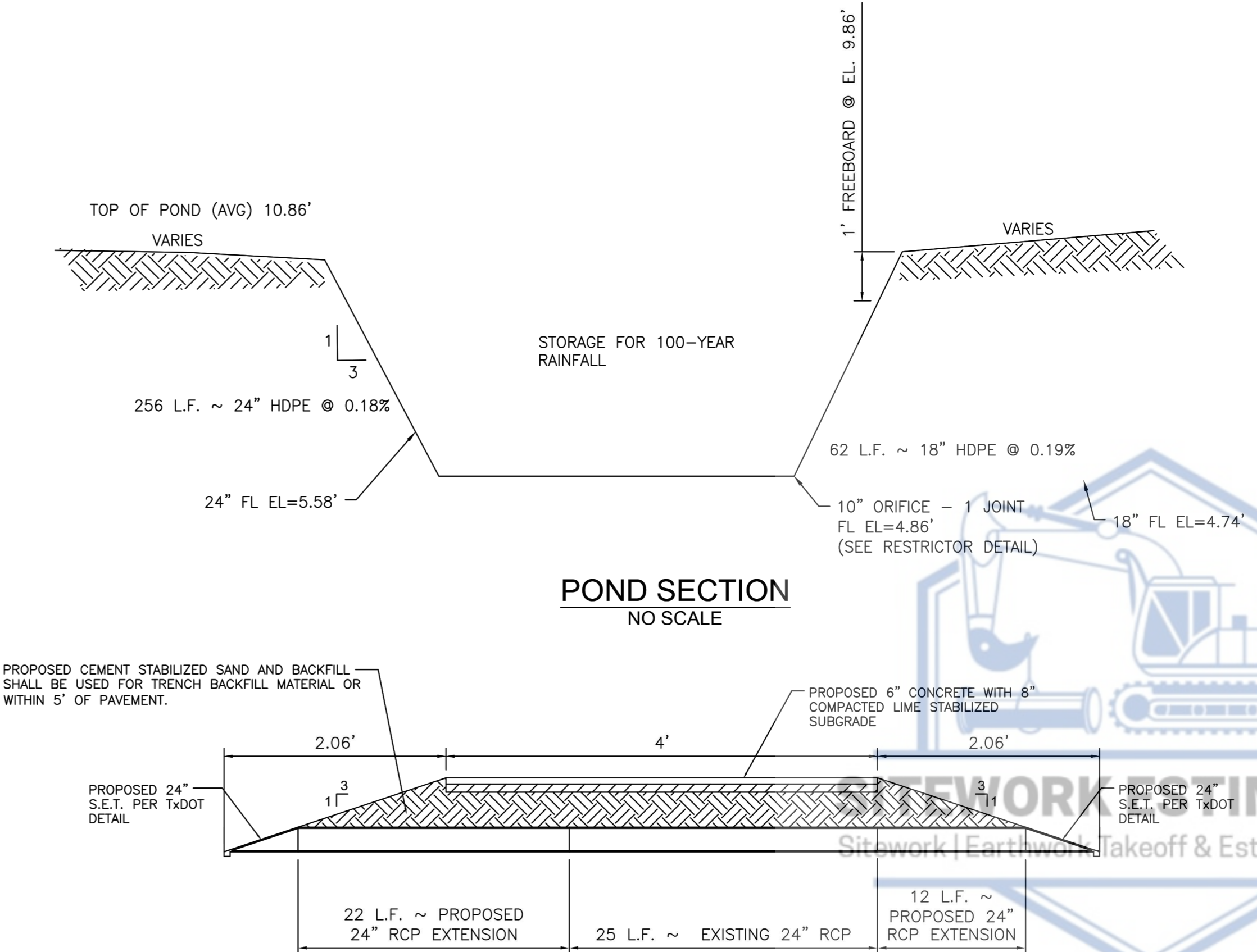
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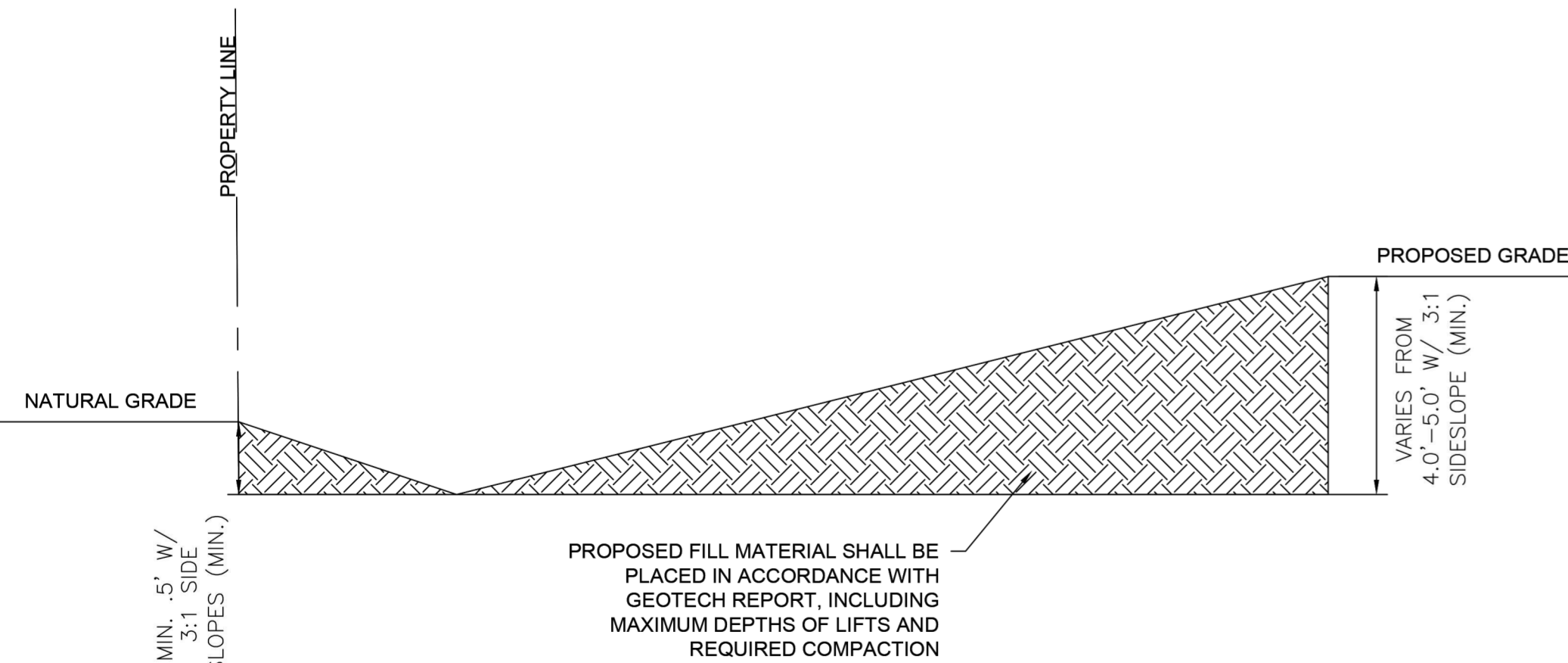
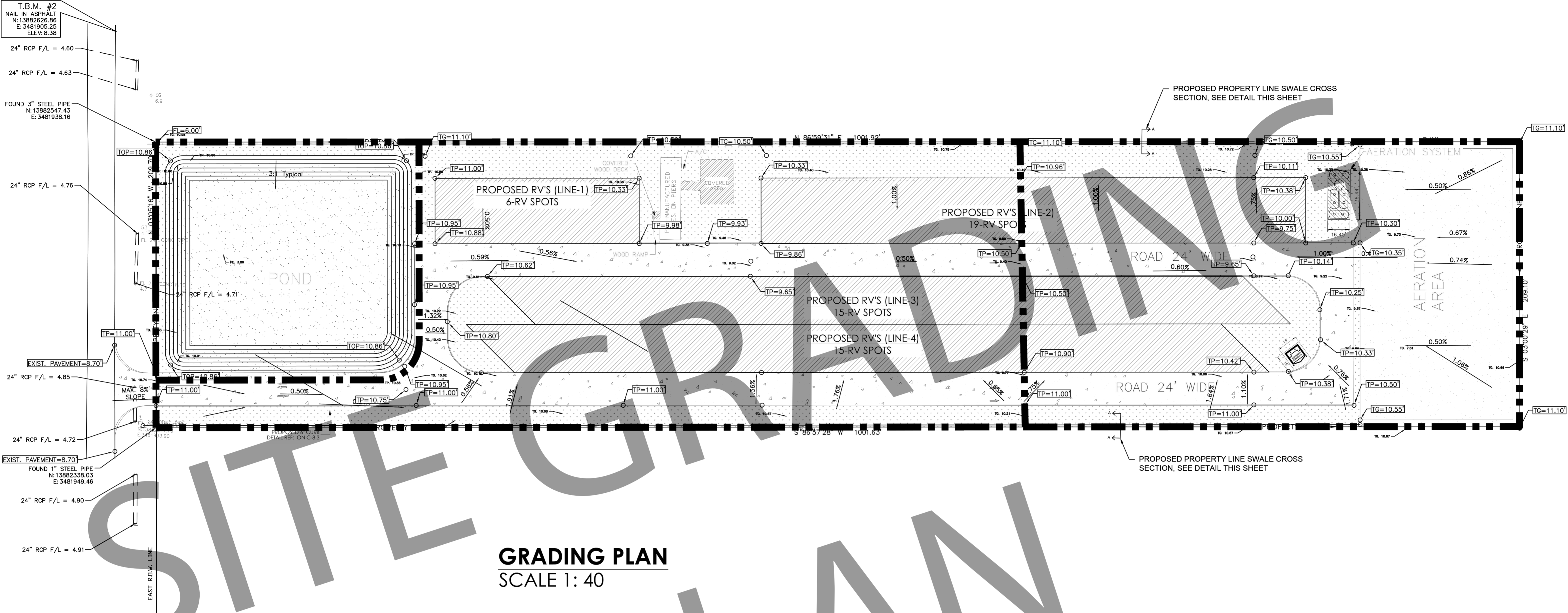
RESTRICTOR DETAIL
NO SCALE



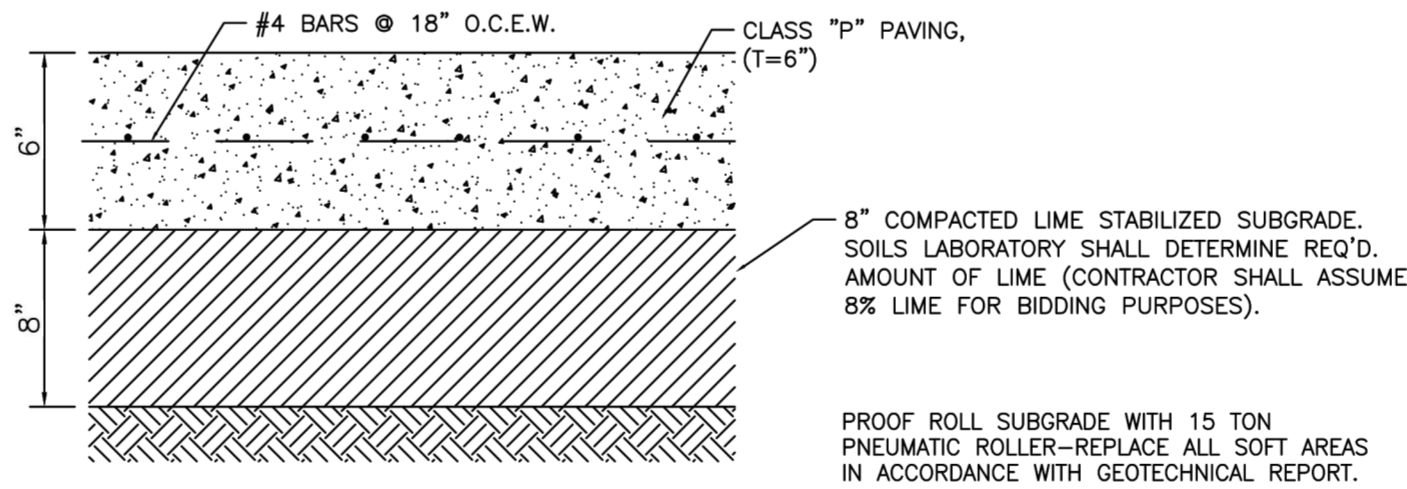
STANDARD GRATE INLET



DRIVEWAY CROSS-SECTION



SECTION A-A PROPOSED
PROPERTY LINE SWALE

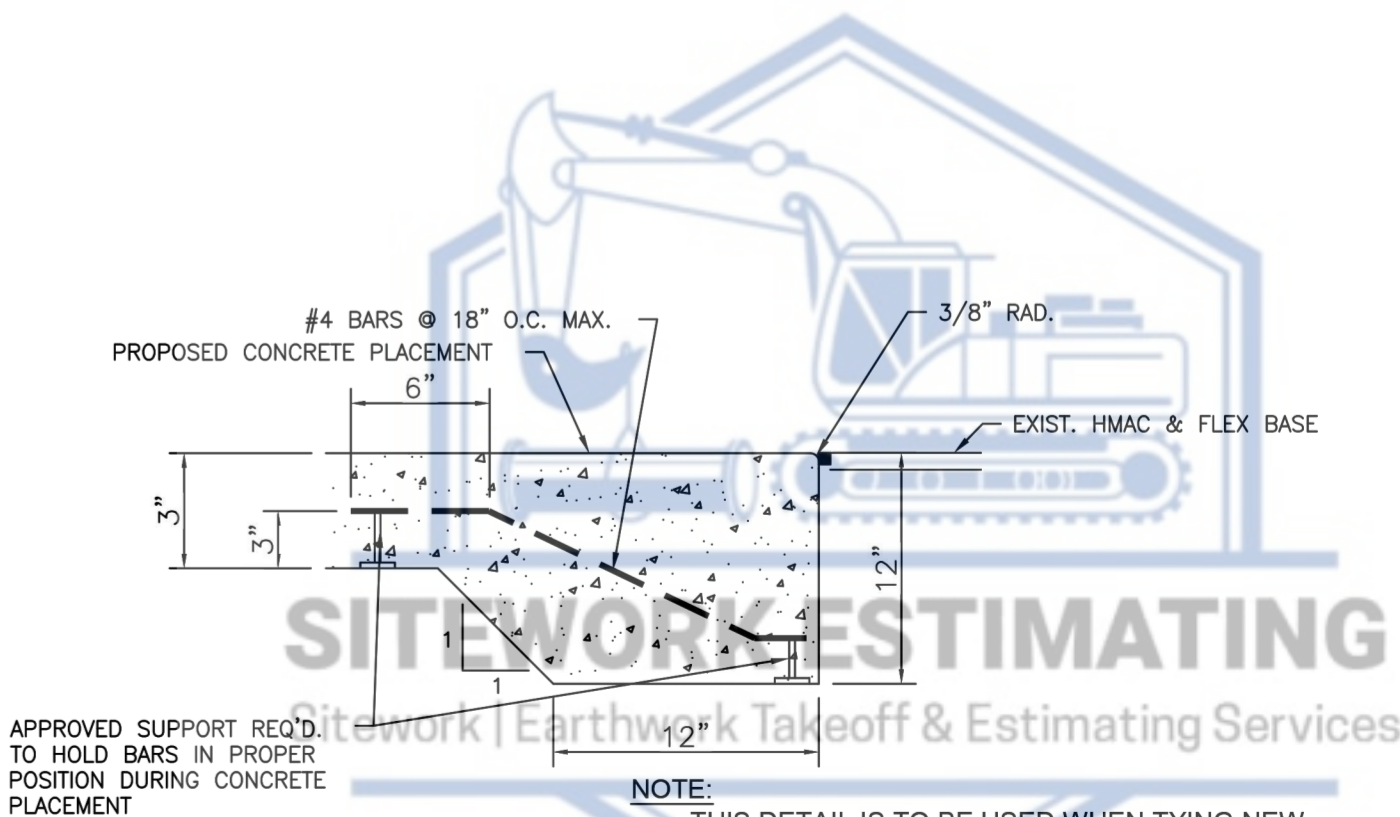


SUBGRADE PREPARATION NOTES

1. THE TOP 6 INCHES OF SOIL, VEGETATION AND ROOTS IN THE PROPOSED PAVEMENT AREAS SHALL BE STRIPPED FROM THE SITE AND EITHER WASTED OR STOCKPILED FOR LATER USE. AFTER STRIPPING AND EXCAVATING TO THE DESIRED SUBGRADE ELEVATION, THE EXPOSED SUBGRADE SOILS SHALL BE PROOF ROLLED WITH AT LEAST A 15 TON PNEUMATIC ROLLER TO DETECT WEAK AREAS. SUCH AREAS SHALL BE REMOVED AND REPLACED WITH SOILS EXHIBITING SIMILAR CLASSIFICATION, MOISTURE CONTENT AND DENSITY AS THE ADJACENT IN SITU SOILS. SUBSEQUENT TO PROOF ROLLING, THE EXPOSED SUBGRADE SHALL BE COMPACTED TO AT LEAST 95 PERCENT OF STANDARD PROCTOR (ASTM D 698) MAXIMUM DRY DENSITY AT OPTIMUM TO +3% ABOVE OPTIMUM MOISTURE CONTENT.
2. ANY FILL REQUIRED FOR GRADING PURPOSES, SHALL BE PLACED IN MAXIMUM LOOSE LIFTS OF 8 INCHES. IF WATER MUST BE ADDED, IT SHALL BE UNIFORMLY APPLIED AND THOROUGHLY MIXED INTO THE SOIL BY DISKING OR SCARIFYING. THE EDGES OF COMPACTED FILL SHALL EXTEND A MINIMUM OF 2 FOOT BEYOND THE EDGES OF THE PAVEMENT PRIOR TO SLOPING. SELECT FILL, IF REQUIRED, SHALL BE FREE OF ORGANIC OR OTHER DELETERIOUS MATERIALS, HAVE A MAXIMUM PARTICLE SIZE LESS THAN 3 INCHES. GEOTECHNICAL REPORT SHALL DETERMINE REQUIRED AMOUNT OF LIME (CONTRACTOR SHALL ASSUME 8% LIME FOR BIDDING PURPOSES). STRUCTURAL SELECT FILL SHALL BE COMPACTED TO AT LEAST 95% OF STANDARD PROCTOR (ASTM D 698 MAXIMUM DRY DENSITY, AT -2% TO +3% OF OPTIMUM MOISTURE CONTENT. STABILIZE THE UPPER 8 INCHES OF SOIL WITH LIME WHEN IN SITU SOILS HAVE PI GREATER THAN 20.

3. REINFORCED CONCRETE SHALL BE CLASS "P" PAVING IN THE THICKNESS SPECIFIED. SEE CONCRETE TABLES.

CONCRETE PAVING SECTION



EXISTING ASPHALT PAVEMENT TRANSITION
(NEW CONC. TO EXIST. ASPHALT)
N.T.S.